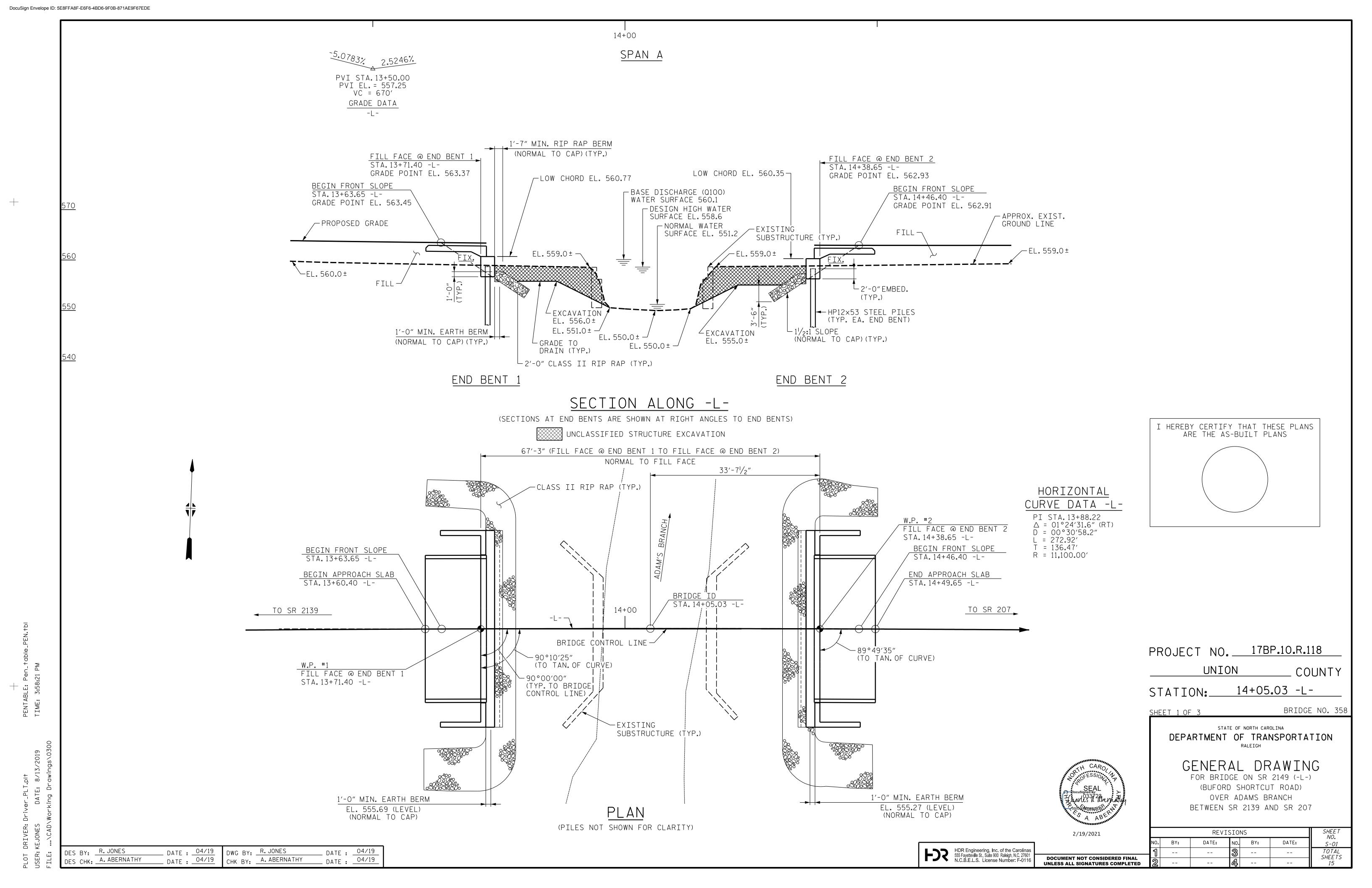
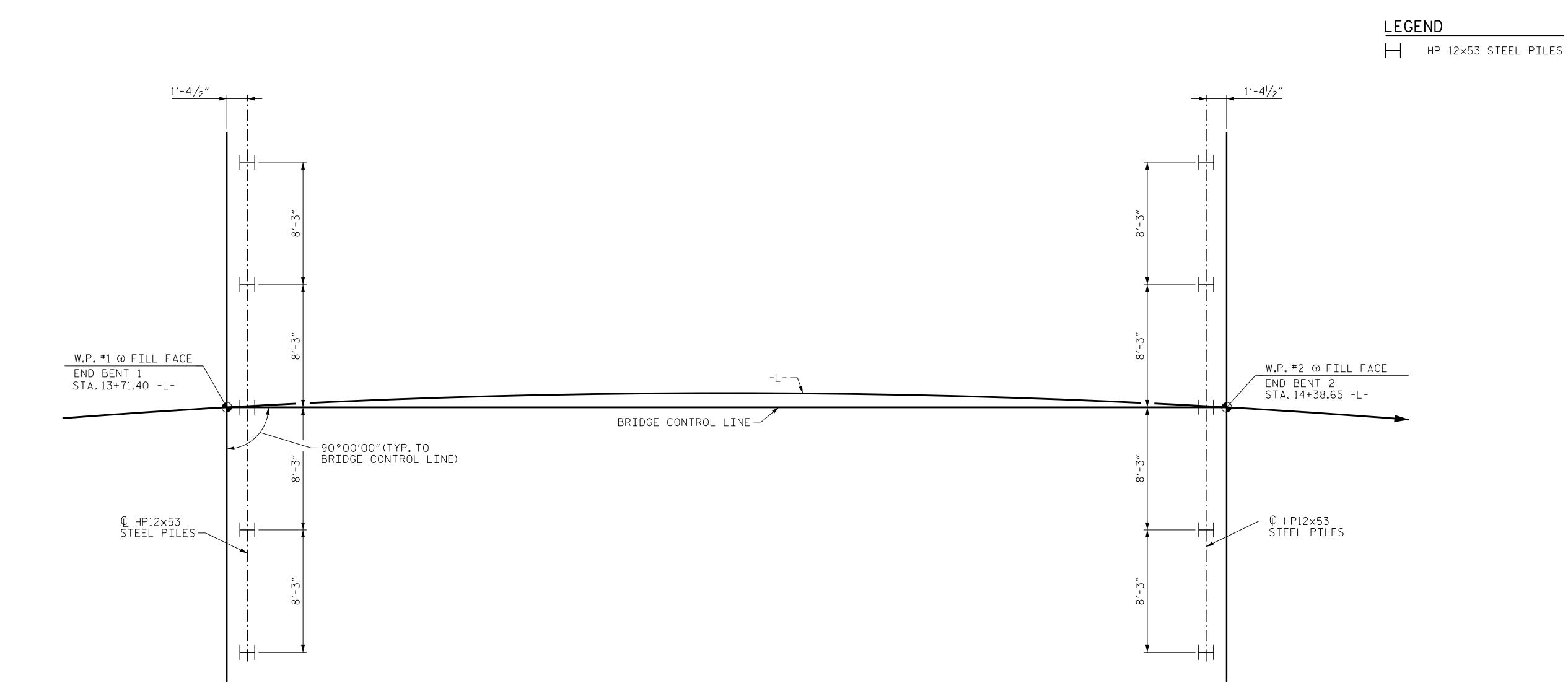
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\_ DATE : 05/19 \_ DATE : 05/19 DWG BY: R. JONES \_ DATE : 05/19 R. JONES DES CHK: A. ABERNATHY CHK BY: A. ABERNATHY \_ DATE : 05/19



# FOUNDATION LAYOUT

NOTES

DIMENSIONS TO PILES ARE MEASURED TO © PILE.

FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENT 1 AND END BENT 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 95 TONS PER PILE.

END BENT 1

DRIVE PILES AT END BENT 1 AND END BENT 2 TO A REQUIRED DRIVING RESISTANCE OF 158 TONS PER PILE.

DRILLED-IN PILES ARE REQUIRED FOR END BENT 1. EXCAVATE HOLES AT PILE LOCATIONS TO ELEVATION 546.2 FT FOR PILE EXCAVATION, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

CONCRETE IS REQUIRED TO FILL HOLES FOR PILE EXCAVATION AT END BENT 1.

STEEL H-PILE POINTS ARE REQUIRED FOR STEEL H-PILES AT END BENT 2.FOR STEEL PILE POINTS, SEE SECTION 450 OF THE STANDARD SPECIFICATION.

PROJECT NO. <u>17BP.10.R.118</u> UNION COUNTY

14+05.03 -L-STATION:\_\_\_

SHEET 2 OF 3

DEPARTMENT OF TRANSPORTATION

STATE OF NORTH CAROLINA

# GENERAL DRAWING

FOR BRIDGE ON SR 2149 (-L-) (BUFORD SHORTCUT ROAD) OVER ADAMS BRANCH BETWEEN SR 2139 AND SR 207

			2/19/2021		
I			ZS1Z ZD1Z	NO.	BY:
	<b>FJ3</b>	HDR Engineering, Inc. of the Carolinas 555 Fayetteville St., Suite 900 Raleigh, N.C. 27601		1	
	1 2 1	N.C.B.E.L.S. License Number: F-0116	DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	2	

END BENT 2

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21		
/D1/	NO.	BY:
	1	

	REVIS	SIO	NS		SHEET NO.
3Y <b>:</b>	DATE:	NO.	BY:	DATE:	S-02
		8			TOTAL SHEETS
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DATE : 05/19

DATE : 05/19

R. JONES

A. ABERNATHY

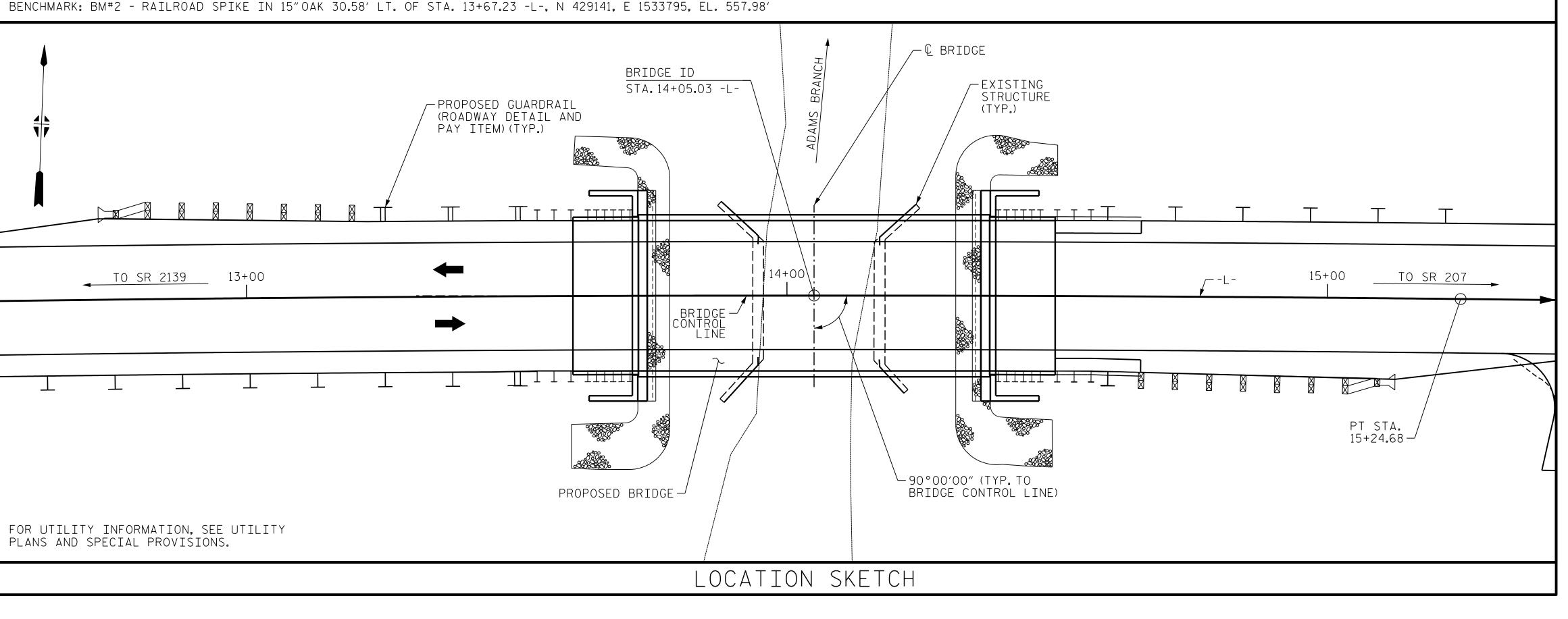
DWG BY: R. JONES

CHK BY: A. ABERNATHY

05/19

DATE:

DATE: 05/19



	TOTAL BILL OF MATERIAL																
	REMOVAL OF EXISTING STRUCTURE AT STATION 14+05.03 -L-	ASBESTOS ASSESSMENT	UNCLASSIFIED STRUCTURE EXCAVATION AT STATION 14+05.03 -L-	CLASS A CONCRETE	BRIDGE APPROACH SLABS AT STATION 14+05.03 -L-	REINF. STEEL	PILE DRIVING EQUIPMENT SETUP FOR HP 12×53 STEEL PILES	HP STEEL	12×53 L PILES	STEEL PILE POINTS	PILE EXCAVATION NOT IN SOIL	PILE EXCAVATION IN SOIL	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	3'-0" X 2'-0" PRESTRESSED CONC. CORED SLABS
	LUMP SUM	LUMP SUM	LUMP SUM	CU. YDS.	LUMP SUM	LBS.	EA.	NO. L	LIN. FT.	EA.	LF	LF	LIN. FT.	TONS	SQ. YDS.	LUMP SUM	NO. LIN. FT.
SUPERSTRUCTURE													130.25				10 650
END BENT NO. 1				20.2		2449	5	5	60	0	20	30		33	37		
END BENT NO. 2				20.2		2449	5	5	75	5	0	0		36	41		
TOTAL	LUMP SUM	LUMP SUM	LUMP SUM	40.4	LUMP SUM	4898	10	10	135	5	20	30	130.25	69	78	LUMP SUM	10 650

HYDRAULIC DATA

DESIGN DISCHARGE = 1,560 CFS FREQUENCY OF DESIGN FLOOD = 25 YR. DESIGN HIGH WATER ELEVATION = 558.6 DRAINAGE AREA = 5.83 SQ. MI. BASE DISCHARGE (Q100) = 2,243 CFS BASE HIGH WATER ELEVATION = 560.1

# OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE = 4,200 CFS FREQUENCY OF OVERTOPPING FLOOD = 500+ YR. OVERTOPPING FLOOD ELEVATION =  $562.9 \Delta$ 

Δ OVERTOPPING OCCURS AT STA. 14+63 -L-

NOTES

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

THIS BRIDGE SHALL BE CONSTRUCTED USING TOP-DOWN CONSTRUCTION METHODS. THE USE OF A TEMPORARY CAUSEWAY OR WORK BRIDGE IS NOT PERMITTED. CRANES SHALL ONLY BE PERMITTED ON SPAN A DURING TOP-DOWN CONSTRUCTION.

FOR OTHER DESIGN DATA AND GENERAL NOTES. SEE "STANDARD NOTES" SHEET.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA ON SHEET S-01 SHALL BE EXCAVATED FOR A DISTANCE OF 40 FT. EACH SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE EXISTING STRUCTURE CONSISTING OF 1 SPAN @ 25'-3" WITH CLEAR ROADWAY OF 19'-1" AND TIMBER DECK ON 9 LINES OF W12X27 STEEL I-BEAMS WITH CONCRETE ABUTMENTS LOCATED AT THE SITE OF THE PROPOSED STRUCTURE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED BELOW THE LEGAL LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, THIS LOAD LIMIT MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER THAT PREVENTS DEBRIS FROM FALLING INTO THE WATER. THE CONTRACTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE THE BRIDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18 -EVALUATING SCOUR AT BRIDGES''.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 14+05.03 -L-.

AT THE CONTRACTOR'S OPTION, PRESTRESSED CONCRETE END BENT AND BENT CAPS MAY BE SUBSTITUTED IN PLACE OF THE CAST-IN-PLACE CAPS. THE CONTRACTOR SHALL COORDINATE WITH THE RESIDENT ENGINEER TO RECEIVE REVISED PLANS AND DETAILSFROM THE STRUCTURES MANAGEMENT UNIT. THE REDESIGN AND ANY ADDITIONAL MATERIALS NEEDED WILL BE AT NO ADDITIONAL COST TO THE CONTRACTOR.

> PROJECT NO. <u>17BP.10.R.118</u> UNION COUNTY 14+05.03 -L-STATION:\_

SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

GENERAL DRAWING

FOR BRIDGE ON SR 2149 (-L-) (BUFORD SHORTCUT ROAD) OVER ADAMS BRANCH BETWEEN SR 2139 AND NC 207

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2/19/2021			REVI	SIO	VS		SHEET NO.
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	11			3			TOTAL
T NOT CONSIDERED FINAL	2			Ā			SHEETS 15

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**DOCUMENT** UNLESS ALL SIGNATURES COMPLETED

DES BY: R. JONES DATE	. 05/19 DWG B	Y: R. JONES	DATE:	05/19
DES CHK: A. ABERNATHY DATE		A A D E D N A T L L V	DATE :	05/19

# LOAD AND RESISTANCE FACTOR RATING (LRFD) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS

							STRENGTH I LIMIT STATE										SE				 Т	TE		
								STRENGTH I LIMIT STATE  MOMENT SHEAR									SERVICE III LIMIT STATE						1	
										MOMENI I	Ι				SHEAR		1			Т	MOMENT	ı		1
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	COMMENT NUMBER
		HL-93(Inv)	N/A		1.018		1.75	0.274	1.05	65′	EL	32	0.513	1.2	65′	EL	6.4	0.80	0.274	1.02	65′	EL	32	
DESIGN		HL-93(0pr)	N/A		1.358		1.35	0.274	1.36	65′	EL	32	0.513	1.56	65′	EL	6.4	N/A						
LOAD RATING		HS-20(Inv)	36.000	2	1.306	47.014	1.75	0.274	1.34	65′	EL	32	0.513	1.48	65′	EL	6.4	0.80	0.274	1.31	65′	EL	32	
IVATINO		HS-20(0pr)	36.000		1.742	62.706	1.35	0.274	1.74	65′	EL	32	0.513	1.92	65′	EL	6.4	N/A						
		SNSH	13.500		2.868	38.725	1.4	0.274	3.69	65′	EL	32	0.513	4.33	65′	EL	6.4	0.80	0.274	2.87	65′	EL	32	
		SNGARBS2	20.000		2.171	43.424	1.4	0.274	2.79	65′	EL	32	0.513	3.11	65′	EL	6.4	0.80	0.274	2.17	65′	EL	32	
		SNAGRIS2	22.000		2.071	45.552	1.4	0.274	2.66	65′	EL	32	0.513	2.89	65′	EL	6.4	0.80	0.274	2.07	65′	EL	32	
		SNCOTTS3	27.250		1.428	38.924	1.4	0.274	1.84	65′	EL	32	0.513	2.17	65′	EL	6.4	0.80	0.274	1.43	65′	EL	32	
		SNAGGRS4	34.925		1.206	42.136	1.4	0.274	1.55	65′	EL	32	0.513	1.81	65′	EL	6.4	0.80	0.274	1.21	65′	EL	32	
		SNS5A	35.550		1.179	41.911	1.4	0.274	1.52	65′	EL	32	0.513	1.85	65′	EL	6.4	0.80	0.274	1.18	65′	EL	32	
		SNS6A	39.950		1.087	43.43	1.4	0.274	1.4	65′	EL	32	0.513	1.69	65′	EL	6.4	0.80	0.274	1.09	65′	EL	32	
LEGAL		SNS7B	42.000		1.035	43.489	1.4	0.274	1.33	65′	EL	32	0.513	1.67	65′	EL	6.4	0.80	0.274	1.04	65′	EL	32	
LOAD RATING		TNAGRIT3	33.000		1.327	43.8	1.4	0.274	1.71	65′	EL	32	0.513	2.01	65′	EL	6.4	0.80	0.274	1.33	65′	EL	32	
INATINO		TNT4A	33.075		1.335	44.142	1.4	0.274	1.72	65′	EL	32	0.513	1.95	65′	EL	6.4	0.80	0.274	1.33	65′	EL	32	
		TNT6A	41.600		1.096	45.613	1.4	0.274	1.41	65′	EL	32	0.513	1.8	65′	EL	6.4	0.80	0.274	1.10	65′	EL	32	
	TST	TNT7A	42.000		1.105	46.4	1.4	0.274	1.42	65′	EL	32	0.513	1.74	65′	EL	6.4	0.80	0.274	1.10	65′	EL	32	
		TNT7B	42.000		1.15	48.298	1.4	0.274	1.48	65′	EL	32	0.513	1.62	65′	EL	6.4	0.80	0.274	1.15	65′	EL	32	
		TNAGRIT4	43.000		1.089	46.815	1.4	0.274	1.4	65′	EL	32	0.513	1.57	65′	EL	6.4	0.80	0.274	1.09	65′	EL	32	
		TNAGT5A	45.000		1.024	46.084	1.4	0.274	1.32	65′	EL	32	0.513	1.57	65′	EL	6.4	0.80	0.274	1.02	65′	EL	32	
		TNAGT5B	45.000	3	1.01	45.431	1.4	0.274	1.3	65′	EL	32	0.513	1.49	65′	EL	6.4	0.80	0.274	1.01	65′	EL	32	

LOAD FACTORS:

DESIGN	LIMIT STATE	$\gamma_{DC}$	$\gamma_{\sf DW}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

3 LEGAL LOAD RATING \*\*

\*\* SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

EL - EXTERIOR LEFT GIRDER ER - EXTERIOR RIGHT GIRDER

> PROJECT NO. <u>17BP.10.R.118</u> UNION \_\_\_\_ COUNTY

> > STATE OF NORTH CAROLINA

STATION: 14+05.03 -L-

DEPARTMENT OF TRANSPORTATION
RALEIGH STANDARD LRFR SUMMARY FOR 65' CORED SLAB UNIT

90° SKEW (NON-INTERSTATE TRAFFIC)

TOTAL SHEETS 15

REVISIONS 2/19/2021

LRFR SUMMARY

HDR Engineering, Inc. of the Carolinas 555 Fayetteville St., Suite 900 Raleigh, N.C. 27601 N.C.B.E.L.S. License Number: F-0116

+







30′-0″ 1'-0" 27'-10"(CLEAR ROADWAY)  $\sim$  VARIES (O"MIN.,  $\frac{5}{8}$ "MAX.) 13'-11" 13'-11" — BRIDGE CONTROL LINE VERTICAL CONCRETE BARRIER RAIL (TYP.) FOR DETAILS SEE "VERTICAL CONCRETE — 7″@ Ĺ BRG. BARRIER RAIL SECTION"  $3\frac{1}{2}$ " @  $\mathbb{L}$  BRG. — — CONST. JT. ASPHALT WEARING GRADE PT.-SURFACE (SEE (TYP.) ROADWAY PLANS) — 0.02 -3<sup>1</sup>/<sub>2</sub>"@ ¢ BRG. 0.02 2'-0" (TYP.) · SHEAR KEYS TO BE FILLED WITH GROUT AFTER 0.6" Ø L.R. TRANSVERSE ALL ERECTION HAS BEEN COMPLETED AND AFTER POST-TENSIONING FINAL TENSIONING OF TRANSVERSE STRANDS STRANDS IN  $2\frac{1}{2}$ " 3'-0" Ø HOLES (TYP.) 15'-0" 15′-0″ 10 PRESTRESSED CONCRETE CORED SLAB UNITS = 30'-0"

HALF SECTION AT INTERMEDIATE DIAPHRAGMS

# YPICAL SECTION

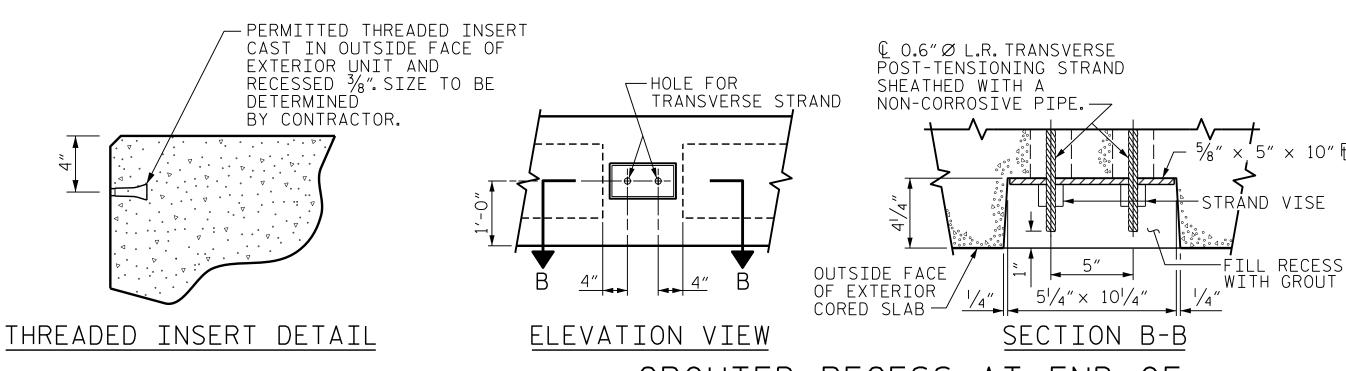
HALF SECTION THROUGH VOIDS

\* - THE MAXIMUM BARRIER RAIL HEIGHT AND ASPHALT THICKNESS IS SHOWN. THE HEIGHT OF THE BARRIER RAIL AND ASPHALT THICKNESS VARIES WHILE THE TOP OF THE BARRIER RAIL FOLLOWS THE PROFILE OF THE GUTTERLINE.

# FIXED END ASPHAL1 WEARING SURFACE . $\sim 2^{1/2}$ Ø DOWEL HOLE 12" Ø VOIDS — SEE "BRIDGE ∠\_\_\_ APPROACH SLAB" SHEET FOR DETAILS — **\_\_\_\_** 2 LAYERS OF 30 LB. ROOFING FELT TO PREVENT BOND. -- ELASTOMERIC 11/2"∅ BACKER ROD BEARING PAD

@ BEARING
& #6 DOWELS

# SECTION AT END BENT

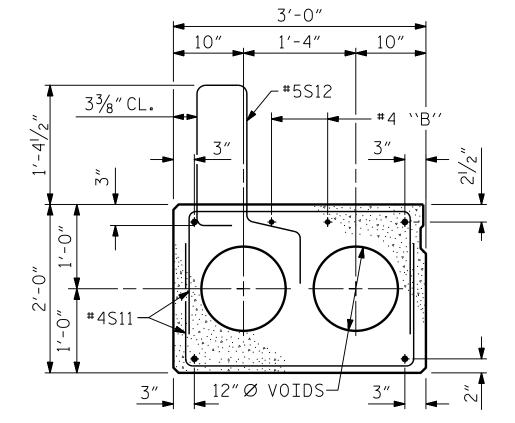


GROUTED RECESS AT END OF POST-TENSIONED STRAND CORED SLABS

-SEE "END BENT"

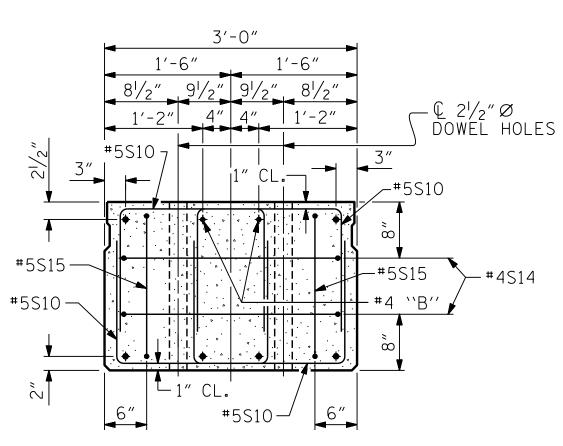
SHEETS FOR DETAILS

DATE : 05/19 DWG BY: R. JONES DATE: 05/19 R. JONES DES BY: DES CHK: A. ABERNATHY CHK BY: A. ABERNATHY DATE : 05/19 \_ DATE : 05/19



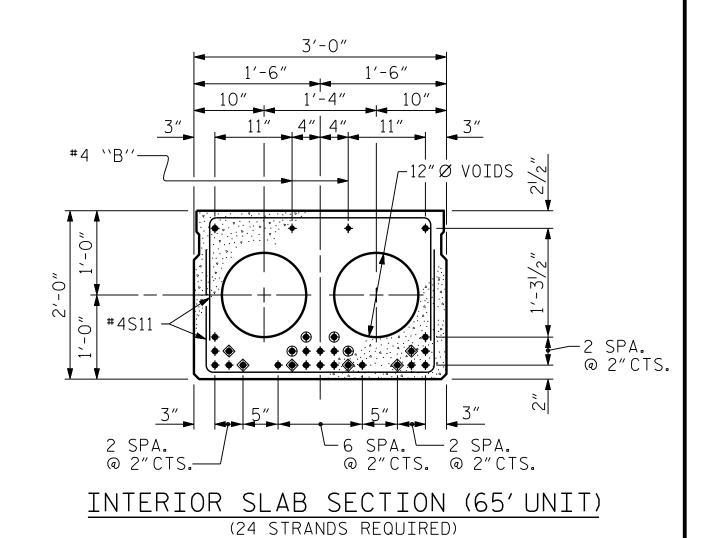
# EXTERIOR SLAB SECTION

(FOR PRESTRESSED STRAND LAYOUT, SEE INTERIOR SLAB SECTION.)



END ELEVATION

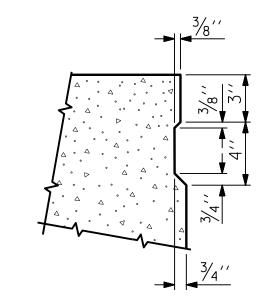
SHOWING PLACEMENT OF DOUBLE STIRRUPS AND LOCATION OF DOWEL HOLES. (STRAND LAYOUT NOT SHOWN.) INTERIOR SLAB UNIT SHOWN-EXTERIOR SLAB UNIT SIMILAR EXCEPT SHEAR KEY LOCATION.



# 0.6'' Ø RELAXATION STRAND LAYOUT

- BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 12'-0" FROM END OF CORED SLAB UNIT. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.
- OPTIONAL FULL LENGTH DEBONDED STRANDS. THESE STRANDS ARE NOT REQUIRED. IF THE FABRICATOR CHOOSES TO INCLUDE THESE STRANDS IN THE CORED SLAB UNIT, THE STRANDS SHALL BE DEBONDED FOR THE FULL LENGTH OF THE UNIT AT NO ADDITIONAL COST. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.

DEBONDING LEGEND



SHEAR KEY DETAIL

NOTE: OMIT SHEAR KEY ON OUTSIDE FACE OF EXTERIOR CORED SLABS.

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PROJECT NO. <u>17BP.10.R.118</u> UNION COUNTY 14+05.03 -L-STATION:



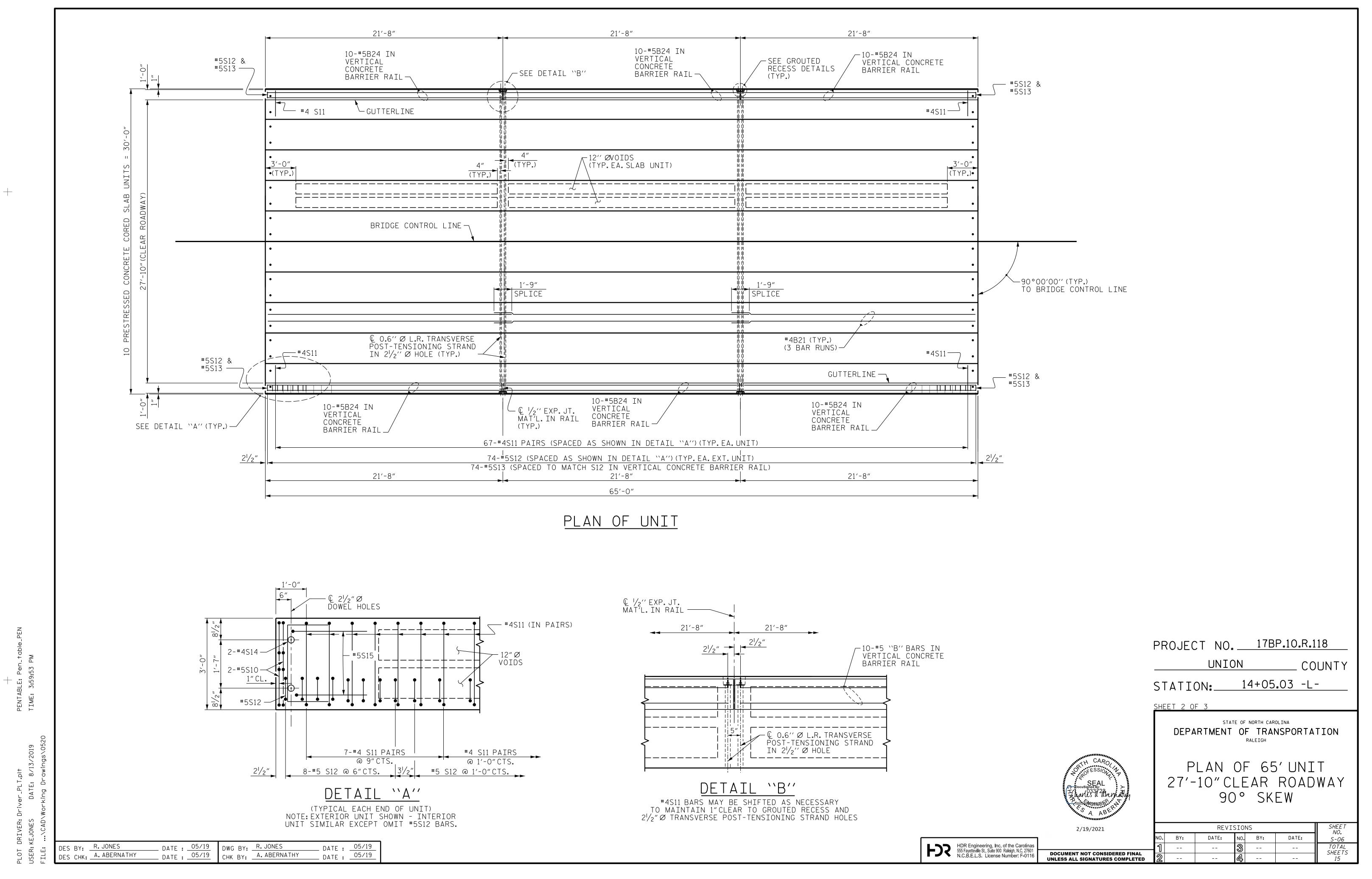
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2/19/2021

SHEET 1 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD 3'-0" X 2'-0" PRESTRESSED CONCRETE CORED SLAB UNIT 90° SKEW

SHEET NO. REVISIONS DATE: S-05 TOTAL SHEETS



## BILL OF MATERIAL FOR VERTICAL CONCRETE BARRIER RAIL BARS PER PAIR OF EXTERIOR UNITS | TOTAL NO. | SIZE | TYPE | LENGTH | WEIGHT 65' UNIT **∗**B24 #5 | STR | 21'-3" | 60 60 1330 **★** S13 148 #5 7′-2″ 148 1106 \* EPOXY COATED REINFORCING STEEL LBS. 2436 CLASS AA CONCRETE CU.YDS. 16.9 130.25 FOTAL VERTICAL CONCRETE BARRIER RAIL LN.FT.

BILL OF MATERIAL FOR ONE 65' CORED SLAB UNIT													
EXTERIOR UNIT   INTERIOR UNIT													
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT						
B21	6	#4	STR	22'-10"	92	22'-10"	92						
S10	8	#5	3	4'-9"	40	4'-9"	40						
S11	134	#4	3	5′-10″	522	5′-10″	522						
* S12	74	#5	1	5′-7″	431								
S14	4	#4	3	5′-7″	15	5′-7″	15						
S15	4	#5	3	7'-1"	30	7'-1"	30						
REINFO	REINFORCING STEEL LBS. 699 699												
* EPOXY COATED													
	REINFORCING STEEL LBS. 431												
6000 F	P.S.I. CO	<u>NCRETE</u>	CU. YDS	) a	11.0		11.0						

No.

24

# BAR TYPES 73/4" 2'-8" 1'-9" ALL BAR DIMENSIONS ARE OUT TO OUT

GUTTERLINE ASPHALT THICKNESS & RAIL HEIGHT ASPHALT OVERLAY THICKNESS RAIL HEIGHT @ MID-SPAN @ MID-SPAN 65'UNIT 2<sup>1</sup>/<sub>8</sub>" 3′-8<sup>l</sup>/<sub>8</sub>″

24

DEAD LOAD DEFLECTION A	AND CAMBER
	3'-0" × 2'-0"
65'CORED SLAB UNIT	O.6"Ø L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	1 7⁄8″ ♦
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD**	1/2″ ♦
FINAL CAMBER	13⁄8″ ♦
** INCLUDES FUTURE WEARING SURFA	CE

CORED	SLABS	s REQ	UIRED
	NUMBER	LENGTH	TOTAL LENGTH
65' UNIT			
EXTERIOR C.S.	2	65′-0″	130′-0″
INTERIOR C.S.	8	65′-0″	520′-0″
$T \cap T \Lambda I$	10		650′-0″

#5S12 & S13

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

PRESTRESSED CONCRETE CORED SLABS.

THE  $2\frac{1}{2}$ " \alpha DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST. AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

ALL REINFORCING STEEL IN VERTICAL CONCRETE BARRIER RAILS SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS,  $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

MAINTAIN A SYMMETRIC TENSION FORCE BETWEEN EACH PAIR OF TRANSVERSE POST TENSIONING STRANDS IN THE DIAPHRAGM.

THE #4 S11 STIRRUPS MAY BE SHIFTED AS NECESSARY TO MAINTAIN 1" CLEAR TO THE GROUTED RECESS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-O"CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

STATION:

SHEET 3 OF 3

	(	GRAD	Ε	270	S	TRAN	DS
						0.6"	ØL
	AREA (SQL	JARE :				0	.217
	ULT] (LBS	MATE S. PER				58	<b>,</b> 600
		IED 9 S.PER				43,	,950
·							
NCRE	ETE	REL	ΕA	SE	S	TREN	IGT

CONCRETE RELEA	ASE STRENGTH
UNIT	PSI
65' UNIT	4800

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UNLESS ALL SIGNATURES COMPLETED

CONCRETE RELEA	ASE STRENGTH
UNIT	PSI
65' UNIT	4800

2/19/2021

(LBS. PER ST	TRAND ) 43,950
NCRETE RELEA	ASE STRENGTH
UNIT	PSI

STATE OF NORTH CAROLINA  DEPARTMENT OF TRANSPORTATION  RALEIGH
STANDARD
3'-0" X 2'-0"
PRESTRESSED CONCRET
CORED SLAB UNIT
90° SKEW
II ous

PROJECT NO. <u>17BP.10.R.118</u>

COUNTY

14+05.03 -L-

UNION

SHEET NO. REVISIONS DATE: DATE: S-07 TOTAL SHEETS

VERTICAL CONCRETE BARRIER RAIL DETAILS

END OF RAIL DETAILS

DWG BY: R. JONES R. JONES A. ABERNATHY

SECTION THRU RAIL

DATE: 05/19 . DATE : 05/19 CHK BY: A. ABERNATHY \_ DATE : 05/19

( TYP.) 101/2 <sub>4</sub> 2 3/8" CL.

CONST. JT. —

<u>'2"CL.</u> <sub>I</sub> MIN.

—#5 S13

- #5S12 SEE "PLAN OF

UNIT" FOR SPACING

SECTION S-S AT DAM IN OPEN JOINT (THIS IS TO BE USED ONLY WHEN SLIP FORM IS USED)  $\frac{1}{2}$ "EXP.JT.MAT'L HELD IN PLACE WITH GALVANIZED NAILS. (NOTE: OMIT EXP. JT. MAT'L. WHEN SLIP FORM IS USED)

CONST.

ELEVATION AT EXPANSION JOINTS

0.6" Ø L.R. STRANDS

FIELD CUT #5S13

FIELD CUT #5S13

FIELD BEND "B" BARS —

CONST. JT.—

2'-0"

6"CTS.

\|FIELD CUT|

[ & S13 @ ] [ & S13 @

6"CTS.

END VIEW

SIDE VIEW

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A  $1/4^{\prime\prime}$  HOLD DOWN PLATE AND 7 -  $1/8^{\prime\prime}$  Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36.AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

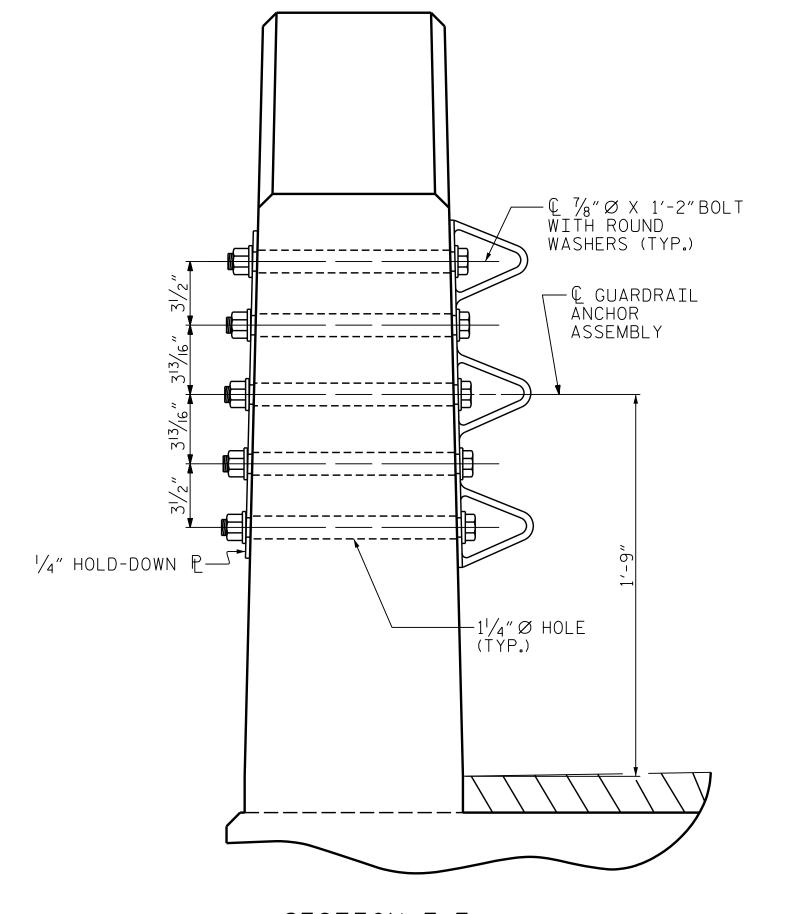
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

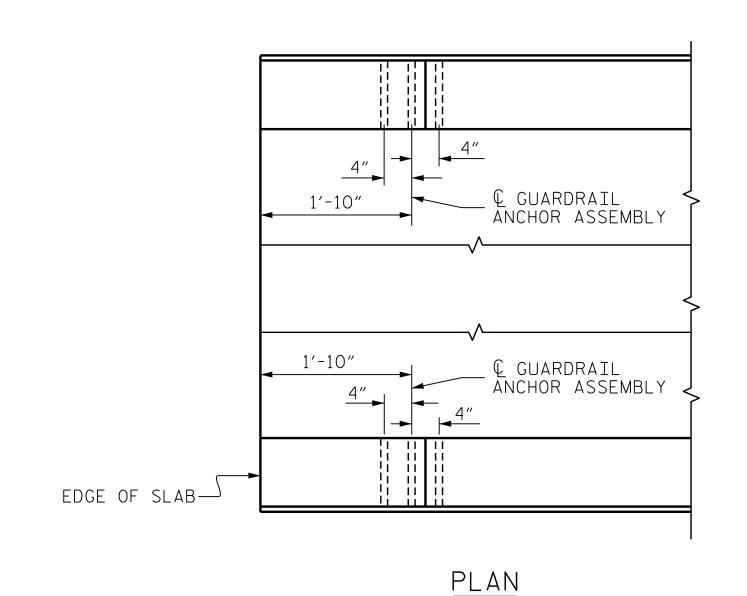
THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1  $\frac{1}{4}$ "  $\varnothing$  HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

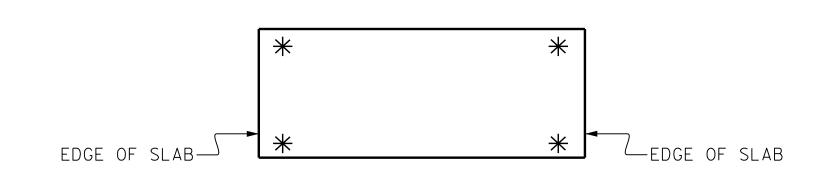


SECTION E-E GUARDRAIL ANCHOR ASSEMBLY DETAILS



LOCATION OF ANCHORS FOR GUARDRAIL

END BENT #1 SHOWN, END BENT #2 SIMILAR.



# SKETCH SHOWING POINTS OF ATTACHMENT

\* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. <u>17BP.10.R.118</u> UNION COUNTY 14+05.03 -L-STATION:\_\_\_

> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION



STANDARD GUARDRAIL ANCHORAGE DETAILS FOR VERTICAL CONCRETE BARRIER RAIL

		` —			
	REVIS	OIS	٧S		SHEET NO.
BY:	DATE:	NO.	BY:	DATE:	S-08
		3			TOTAL SHEETS
		<u>a</u> j			15



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DWG BY: R. JONES \_ DATE : 05/19 \_ DATE : 05/19 R. JONES DES CHK: A. ABERNATHY DATE: 05/19 CHK BY: A. ABERNATHY

NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

> THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4.

PROJECT NO. <u>17BP.10.R.118</u>

UNION COUNTY

14+05.03 -L-STATION:

SHEET 1 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION SUBSTRUCTURE

END BENT No. 1

SHEET NO. S-09 REVISIONS DATE: DATE: TOTAL SHEETS 15

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2/19/2021

DWG BY: R. JONES DATE: 05/19 \_ DATE : 05/19 \_ DATE : 05/19 R. JONES DES BY: DES CHK: A. ABERNATHY CHK BY: A. ABERNATHY \_ DATE : 05/19

FOR WING DETAILS, SEE SHEET 3 OF 4.

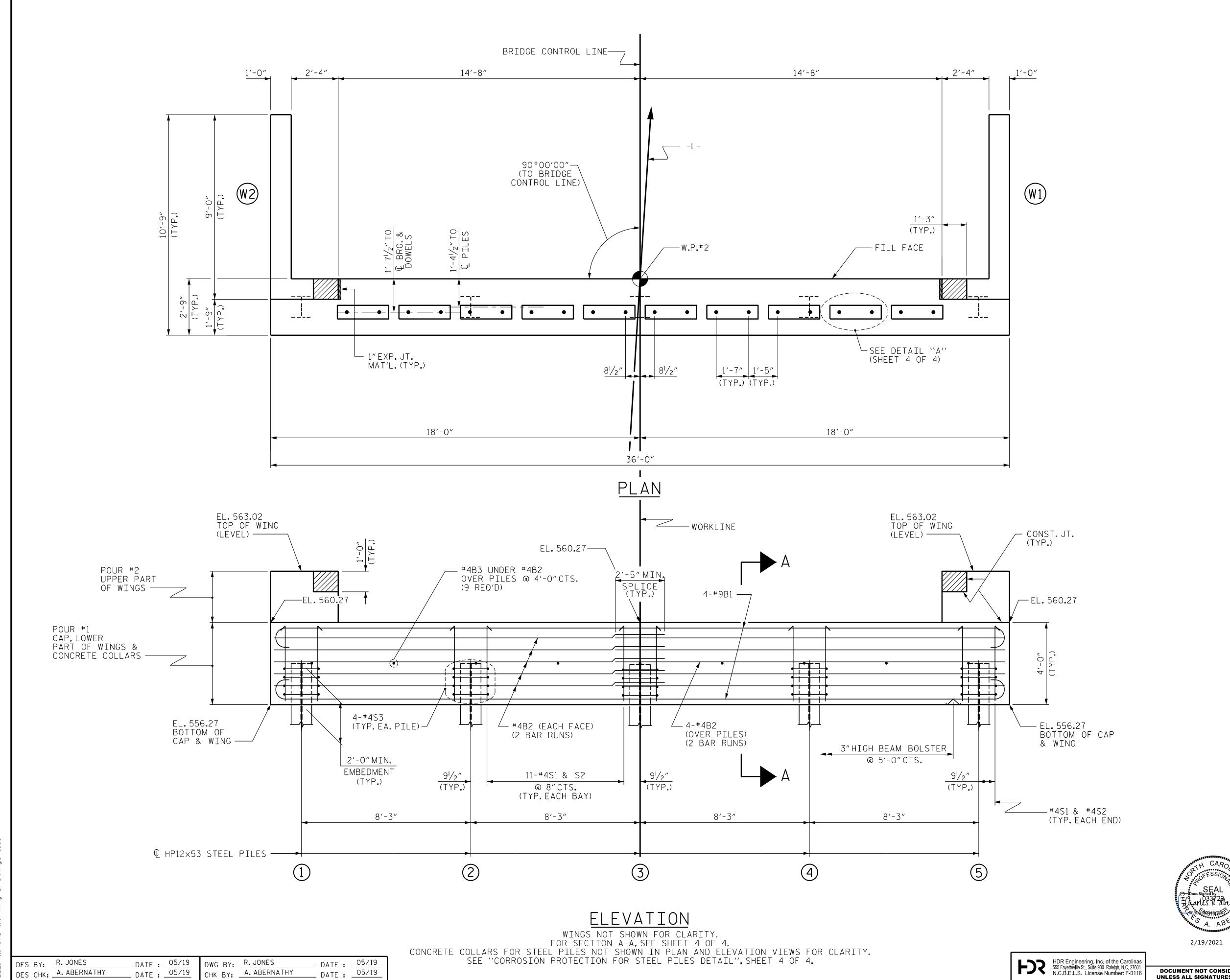
CONCRETE COLLARS FOR STEEL PILES NOT SHOWN IN PLAN AND ELEVATION VIEWS FOR CLARITY.

SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL", SHEET 4 OF 4.

DES CHK: A. ABERNATHY

CHK BY: A. ABERNATHY

\_ DATE : 05/19



NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4. FOR WING DETAILS, SEE SHEET 3 OF 4.

PROJECT NO. <u>17BP.10.R.118</u>

UNION COUNTY

14+05.03 -L-STATION:

SHEET 2 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION SUBSTRUCTURE

END BENT No. 2

SHEET NO. S-10

TOTAL SHEETS 15

DATE:

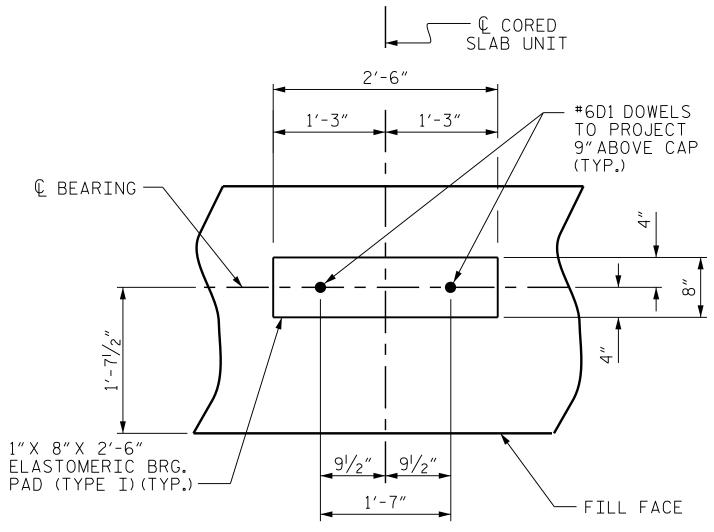
REVISIONS DATE: DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

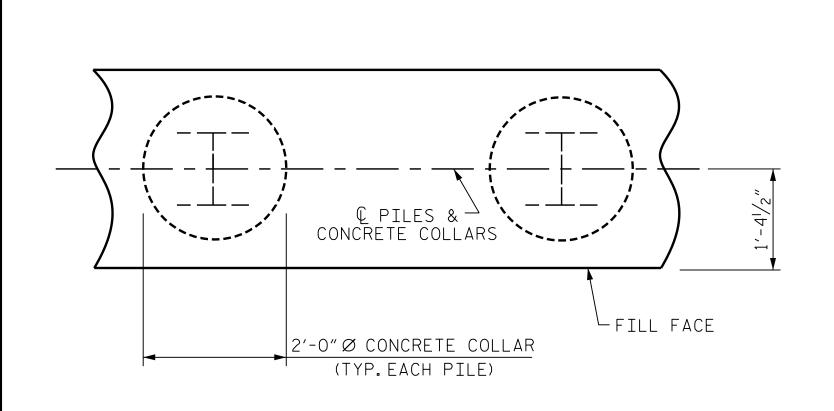
BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

# TEMPORARY DRAINAGE AT END BENT





CORROSION PROTECTION FOR STEEL PILES DETAIL

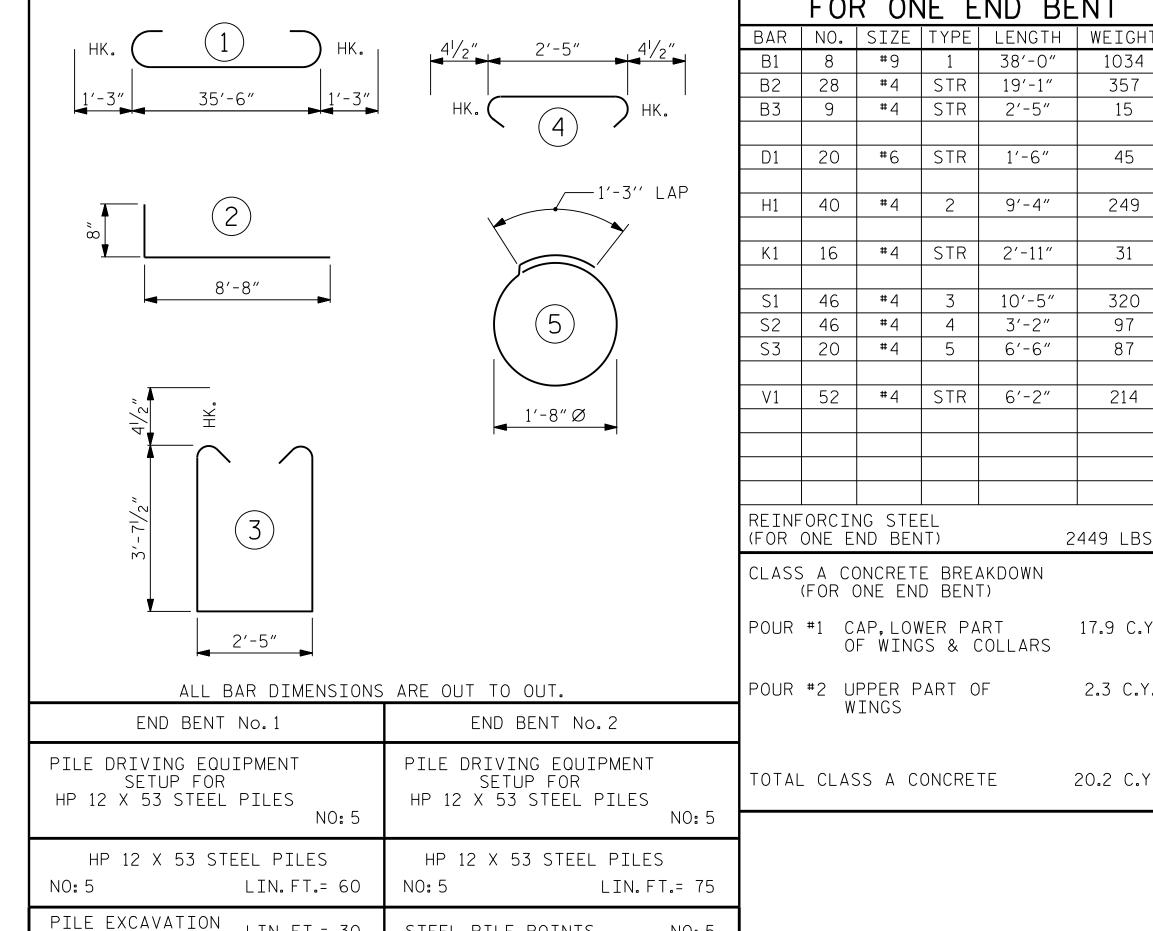
(END\_BENT\_No.1 SHOWN, END\_BENT\_No.2 SIMILAR\_BY\_ROTATION)

DATE: 05/19

DATE: 05/19

/ BACK GOUGE DETAIL B PILE HORIZONTAL OR VERTICAL V∏ 0′′ TO 1/8′′ 0 L 0'' TO 1/8'' DETAIL A DETAIL B POSITION OF PILE DURING WELDING.

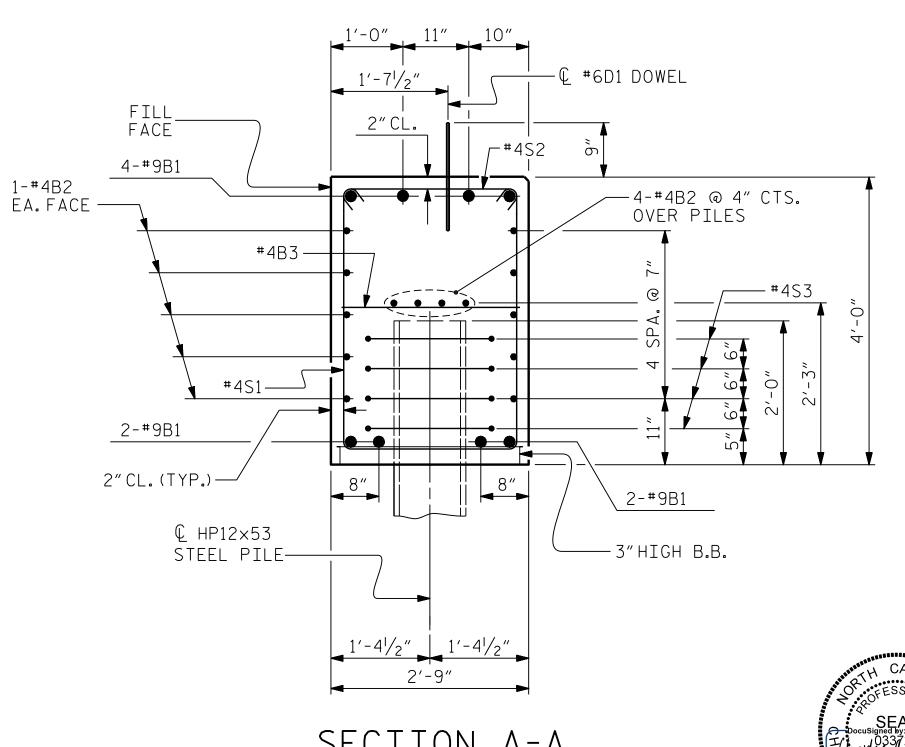
PILE SPLICE DETAILS



LIN.FT.= 30

STEEL PILE POINTS

BAR TYPES



IN SOIL

NOT IN SOIL

PILE EXCAVATION LIN. FT.= 20

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2/19/2021

END BENT No.1 & 2 DETAILS

> SHEET NO. REVISIONS DATE: DATE: TOTAL SHEETS

SECTION A-A (CONCRETE COLLAR NOT SHOWN FOR CLARITY. SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL."

DATE: 05/19 DWG BY: R. JONES R. JONES DES CHK: A. ABERNATHY CHK BY: A. ABERNATHY DATE: 05/19

PLAN

DETAIL "A"

(END BENT No.1 SHOWN, END BENT No.2 SIMILAR BY ROTATION)

CONCRETE COLLAR ---BOTTOM OF CAP © HP12×53 STEEL PILE 2'-0" ELEVATION

**DOCUMENT NOT CONSIDERED FINAL** 

UNLESS ALL SIGNATURES COMPLETED

REINFORCING STEEL (FOR ONE END BENT) 2449 LBS

NO: 5

(FOR ONE END BENT) POUR #1 CAP, LOWER PART 17.9 C.Y. OF WINGS & COLLARS

BILL OF MATERIAL

#9 | 1 |

#4 | STR |

#4 | STR |

#6 | STR |

#4 | STR |

#4 | STR |

4

5

#4

#4

#4

#4

28

40

16

46

52

FOR ONE END BENT

38′-0″

19'-1"

2'-5"

1′-6″

9′-4″

2'-11"

10′-5″

3′-2″

6′-6″

6′-2″

357

15

45

249

31

320

97

87

214

2.3 C.Y.

20.2 C.Y.

TOTAL CLASS A CONCRETE

WINGS

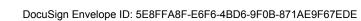
PROJECT NO. <u>17BP.10.R.118</u>

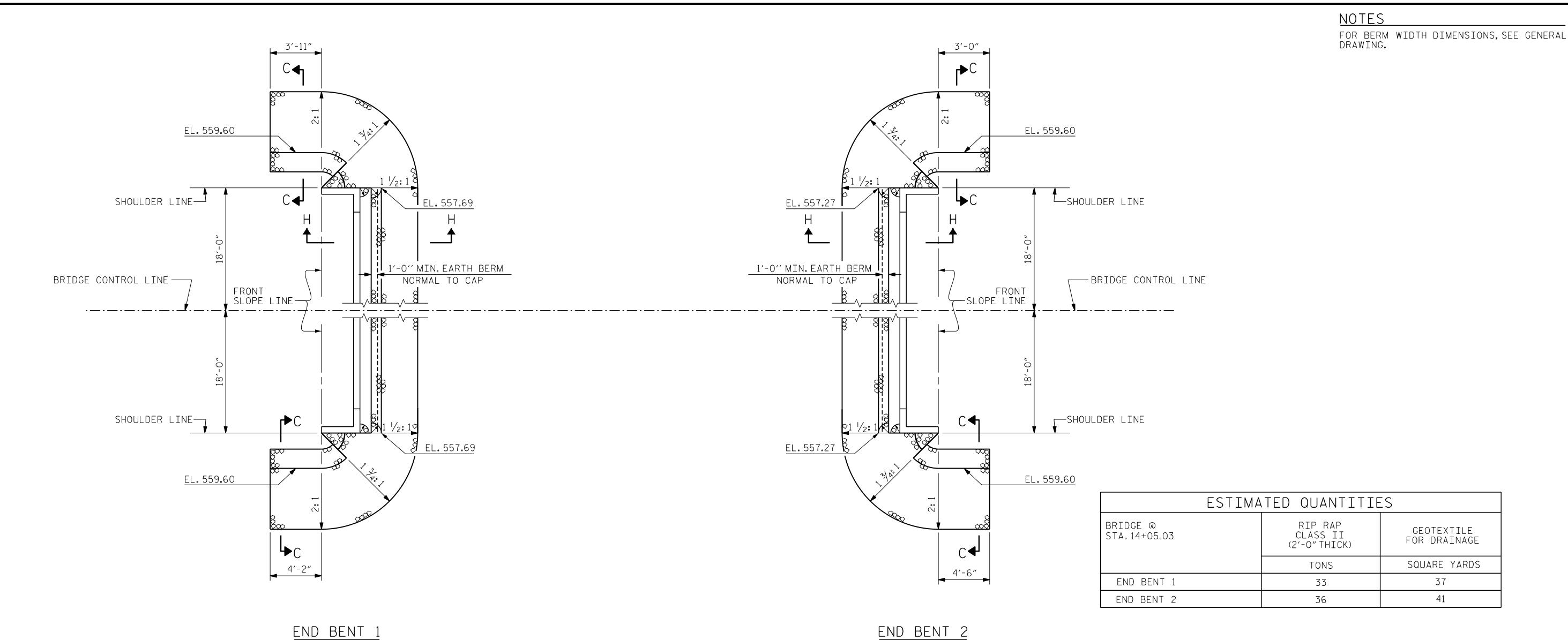
UNION COUNTY

14+05.03 -L-STATION:\_

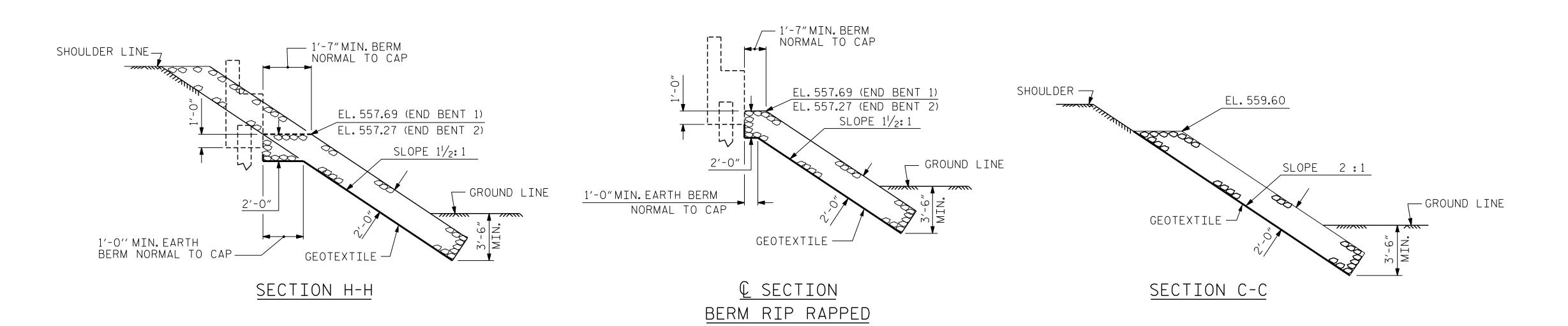
SHEET 4 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION SUBSTRUCTURE









PROJECT NO. <u>17BP.10.R.118</u> UNION COUNTY

14+05.03 -L-STATION:\_\_

> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
> RALEIGH STANDARD

RIP RAP DETAILS

	2/19/2021		REVISIONS					SHEET NO.
		NO.	BY:	DATE:	NO.	BY:	DATE:	S-13
ic. of the Carolinas 00 Raleigh, N.C. 27601		1			3			TOTAL SHEETS
e Number: F-0116	DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	2			4			15

DES BY: R. JONES
DES CHK: A. ABERNATHY DWG BY: R. JONES
CHK BY: A. ABERNATHY \_ DATE : 05/19 \_ DATE : 05/19 \_ DATE : 05/19 \_ DATE : 05/19

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+

SPECIFICATIONS SECTION 1056.

GEOTEXTILE SHALL BE TYPE 1 IN ACCORDANCE WITH THE STANDARD

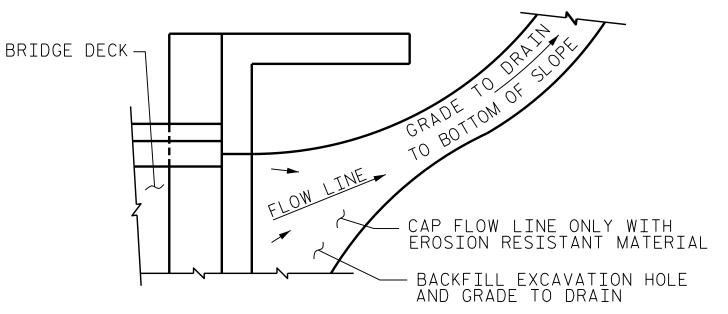
SELECT MATERIAL BACKFILL (CLASS V OR CLASS VI) SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS SECTION 1016.

SELECT MATERIAL BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF BACKWALL FROM OUTSIDE EDGE TO OUTSIDE EDGE OF APPROACH SLAB.

FOR THE 4" Ø DRAINAGE PIPE OUTLET(S), SEE ROADWAY STANDARD DRAWINGS.

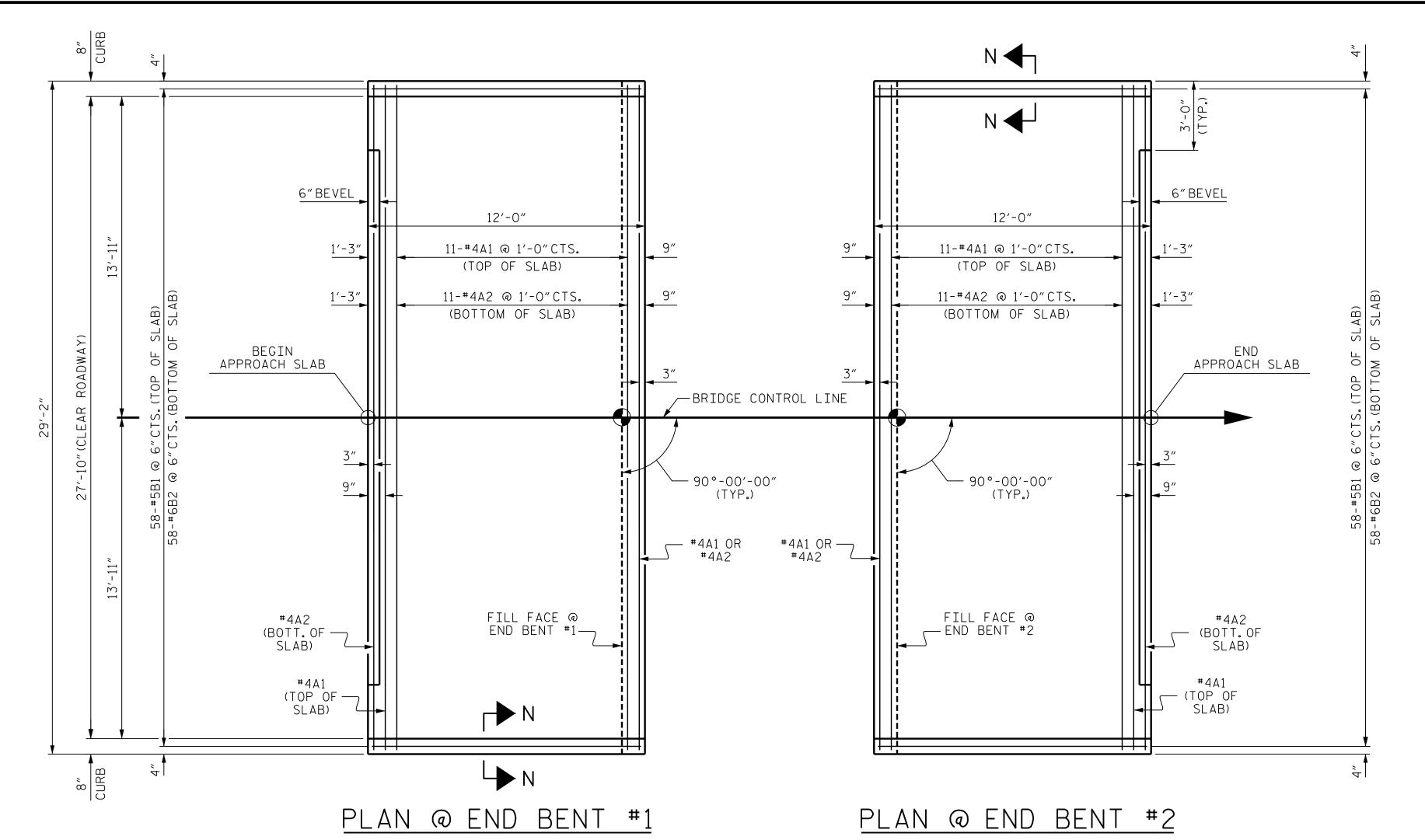
AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALLBE PAVED. SEE ROADWAY PLANS.

APPROACH SLAB GROOVING IS NOT REQUIRED.

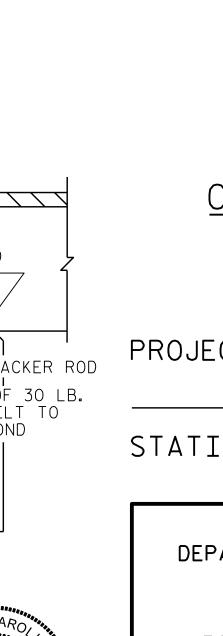


IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

TEMPORARY DRAINAGE DETAIL



DIMENSIONS SHOWN ARE TYPICAL FOR BOTH APPROACH SLABS



SECTION N-N

BILL OF MATERIAL

APPROACH SLAB AT EB #1

BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT

APPROACH SLAB AT EB #2

STR

13 | #4 | STR | 28'-10"

STR 28'-10"

LBS.

LBS.

C.Y.

11'-8"

LBS.

LBS.

C.Y.

676

1016

1266

| WEIGHT

250

676

1016

1266

#4

\*B1 | 58 | #5 | STR | 11'-2"

B2 | 58 | #6 | STR | 11'-8"

BAR | NO. | SIZE | TYPE | LENGTH

\* A1 | 13 | #4 | STR | 28'-10"

\*B1 58 #5 STR 11'-2"

58 #6

REINFORCING STEEL

CLASS AA CONCRETE

REINFORCING STEEL

\* EPOXY COATED

A2 | 13 | #4 | STR | 28'-10"

13

REINFORCING STEEL

CLASS AA CONCRETE

REINFORCING STEEL

\* EPOXY COATED

CURB DETAILS

PROJECT NO. <u>17BP.10.R.118</u>

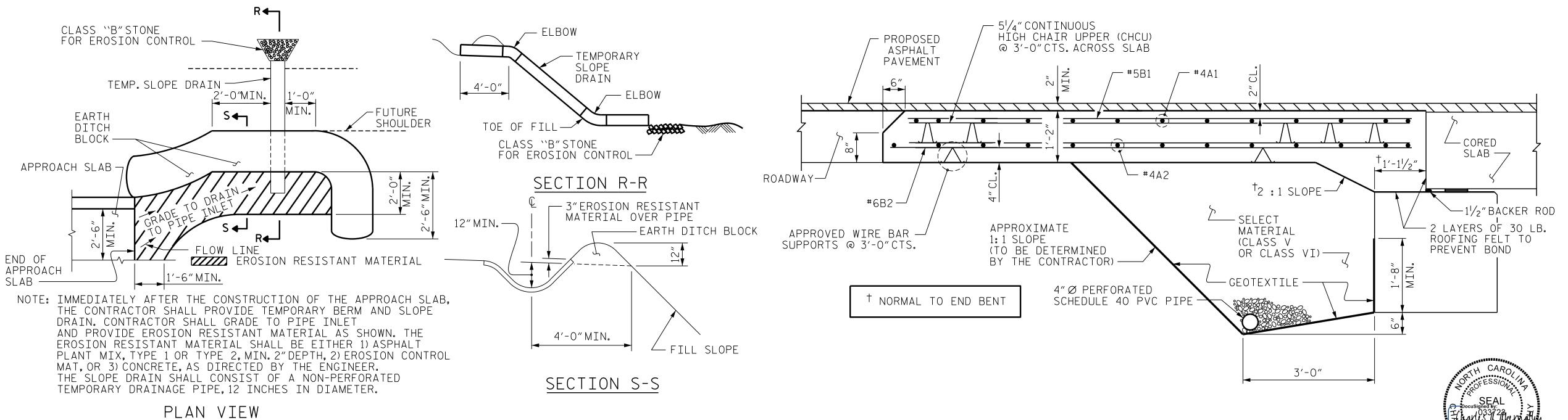
UNION COUNTY

14+05.03 -L-STATION:

> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

STANDARD BRIDGE APPROACH SLAB FOR PRESTRESSED CONCRETE CORED SLAB UNIT (SUB-REGIONAL TIER)

> SHEET NO. REVISIONS DATE: DATE: BY: S-14 TOTAL SHEETS



TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)

DATE : 05/19 DWG BY: R. JONES DATE: 05/19 R. JONES DES BY: DES CHK: A. ABERNATHY CHK BY: A. ABERNATHY DATE : 05/19 DATE: 05/19 HDR Engineering, Inc. of the Carolinas 555 Fayetteville St., Suite 900 Raleigh, N.C. 27601 N.C.B.E.L.S. License Number: F-0116

SECTION THRU SLAB

(TYPE II - MODIFIED APPROACH FILL)

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**UNLESS ALL SIGNATURES COMPLETED** 

2/19/2021

# STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT,

# DESIGN DATA:

SPECIFICATIONS A.A.S.H.T.O. (CURRENT)
LIVE LOAD
IMPACT ALLOWANCE SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W 27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50 27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION - GRADE 60 24,000 LBS. PER SQ. IN.
CONCRETE IN COMPRESSION
CONCRETE IN SHEAR SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS 1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER 375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH 30 LBS.PER CU.FT. (MINIMUM)

# MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

# CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

# CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED  $rac{3}{4}$ " with the following exceptions: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2" RADIUS WHICH IS BUILT INTO CURB FORMS: CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A  $\frac{1}{4}$  FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

# DOWELS:

R. JONES

DES BY:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

# ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS. CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

# REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

# STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE  $\frac{1}{2}$ "  $\varnothing$  SHEAR STUDS FOR THE  $\frac{3}{4}$ " Ø studs specified on the plans. This substitution shall be made at THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 1/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR  $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 -  $\frac{7}{8}$ " Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST \( \frac{5}{6}'' \) IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY  $V_{16}$ INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

# HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

# SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

> PROJECT NO. \_\_\_\_17BP.10.R.118 UNION COUNTY 14+05.03 -L-STATION:\_

2/19/2021

STANDARD NOTES

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

REVISIONS DATE: DATE: S-15 TOTAL SHEETS

DATE : 05/19 DWG BY: R. JONES DATE : \_\_05/19 DES CHK: A. ABERNATHY CHK BY: A. ABERNATHY . DATE : 05/19 DATE: 05/19 HDR Engineering, Inc. of the Carolinas 555 Fayetteville St., Suite 900 Raleigh, N.C. 27601 N.C.B.E.L.S. License Number: F-0116

**DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED**